GENERAL NOTES AND STRUCTURAL SPECIFICATIONS

1. GENERAL NOTES

1.1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, ENGINEER, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE STRUCTURAL ENGINEER OF RECORD OR ANY OF THE STRUCTURAL ENGINEER OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.

1.2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.

1.3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.

1.4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE MORE STRINGENT REQUIREMENT SHALL GOVERN.

1.5. COPIES OF THE SITE SUBSURFACE INVESTIGATION WILL BE MADE AVAILABLE UPON REQUEST. THE OWNER WILL NOT BE RESPONSIBLE FOR INTERPRETATION OR CONCLUSIONS DRAWN THEREFROM BY THE CONTRACTOR. THE DATA IS MADE AVAILABLE FOR THE CONVENIENCE OF THE CONTRACTOR AND IS NOT GUARANTEED TO REPRESENT ALL CONDITIONS THAT MAY BE OCCURRED.

1.6. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.

1.7. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.

1.8. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. OWNER/ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY.

1.9. CONTRACTOR SHALL HAVE THE STRUCTURE LOCATION STAKED AND CERTIFIED BY A LICENSED SURVEYOR. IF DISCREPANCIES BETWEEN ACTUAL LINES AND ELEVATIONS EXIST, NOTIFY STRUCTURAL ENGINEER BEFORE PROCEEDING WITH LAYOUT OF STRUCTURE.

1.10. CONTRACTOR SHALL VERIFY THE STRUCTURALLY SUPPORTED MECHANICAL EQUIPMENT WEIGHTS, OPENING SIZES AND LOCATIONS IDENTIFIED ON THE STRUCTURAL DRAWINGS WITH MECHANICAL DRAWINGS.

1.11. CONTRACTOR SHALL VERIFY THAT MISCELLANEOUS FRAMING SHOWN ON THE STRUCTURAL DRAWINGS FOR MECHANICAL EQUIPMENT, OWNER-FURNISHED ITEMS, PARTITIONS, ETC. IS CONSISTENT WITH THE REQUIREMENTS OF SUCH ITEMS.

1.12. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.

1.13. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR.

1.14. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.

1.15. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.

1.16. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.

1.17. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THE TYPICAL DETAILS UNLESS THOSE LOCATIONS ARE SPECIFICALLY DETAILED OTHERWISE.

1.18. SCALING OF DRAWINGS SHALL NOT BE USED TO OBTAIN OR VERIFY ANY DIMENSION SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL REFER TO THE ENGINEER FOR ANY DIMENSION NOT PROVIDED ON THE DRAWINGS.

2. CODE/DESIGN CRITERIA

2.1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, 2018 EDITION.

2.2. UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE), UNLESS NOTED OTHERWISE IN THE DRAWINGS:

2.2.1.	CATWALKS AND ACCESS PLATFORMS	75	PSF
2.2.2.	PIPING ALLOWANCE	20	PSF
2.2.3.	STAIRS	100	SF
2.2.4.	MANUFACTURING HEAVY	250	SF

2.3. UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

2.3.1.	ROOF	20	SF

2.4. SNOW LOADS

2.4.1.	GROUND SNOW LOAD	P_G = 30 PSF
2.4.2.	SNOW EXPOSURE COEFFICIENT	C_E = 1.0
2.4.3.	SNOW LOAD IMPORTANCE FACTOR	I = 1.1 (RISK CAT III)
2.4.4.	THERMAL FACTOR	C_T= 1.2

2.4.5. PONDING AND DRIFT EFFECTS HAVE BEEN INCLUDED IN THE DESIGN.

2.5. CONCENTRATED FLOOR LOADS - DISTRIBUTED OVER AN AREA OF 2-1/2 SQUARE FEET, UNLESS NOTED OTHERWISE:

2.5.1.	CATWALK AND ACCESS PLATFORMS	300	LB
2.5.2.	MANUFACTURING HEAVY	3000	LB

2.6. DEAD LOADS (IN ADDITION TO STRUCTURE SELF-WEIGHT):

2.6.1.	PIPING ALLOWANCE	20	PSF
2.6.2.	GRATING	15	PSF
2.6.3.	ROOFING	10	PSF

2.7. WIND LOADS:

2.7.1.	BASIC WIND SPEED	117 MPH
2.7.2.	EXPOSURE,	C
2.7.3.	RISK CATEGORY	II
2.7.4.	INTERNAL PRESSURE COEFFICIENT	
2.7.5.	ENCLOSED BUILDINGS	GC_{pi}= +/- 0.18
2.7.6.	SEE COMPONENT AND CLADDING DESIGN WIND PRESSURE DIAGRAM	

2.8. EARTHQUAKE LOADS:

2.8.1.	OCCUPANCY CATEGORY,	F-1
2.8.2.	SEISMIC IMPORTANCE FACTOR,	1.0
2.8.3.	MAPPED SPECTRAL ACCELERATION FOR SHORT PERIODS	S_S =0.197
2.8.4.	MAPPED SPECTRAL ACCELERATION FOR A 1 SECOND PERIOD	S_1 =0.083
2.8.5.	SHORT PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT	S_{DS} =0.170
2.8.6.	1 SECOND PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT	S_{D1} =0.083
2.8.7.	SITE CLASS C (REFER TO GEOTECHNICAL REPORT)	
2.8.8.	SEISMIC DESIGN CATEGORY B	
2.8.9.	BASIC SEISMIC-FORCE RESISTING SYSTEM: STEEL ORDINARY CONCENTRICALLY BRACED FRAMES AND ORDINARY REINFORCED MASONRY SHEAR WALLS	
2.8.10.	SEISMIC RESPONSE COEFFICIENT	CS =0.044
2.8.11.	SEISMIC MODIFICATION FACTOR	R = 3.0
2.8.12.	SEISMIC BASE SHEAR	V=278.6 KIPS

2.9. UNLESS NOTED OTHERWISE CALCULATED INDIVIDUAL MEMBER DEFLECTIONS (IN INCHES) DO NOT EXCEED THE FOLLOWING:

	LIVE LOAD	SNOW/WIND LOAD	DEAD + LIVE LOAD
ROOF MEMBERS:	L/240	L/240	L/180
FLOOR MEMBERS:	L/360		L/240

WHERE, L = SPAN LENGTH (IN INCHES) BETWEEN SUPPORTS. (FOR CANTILEVERS, L IS TWICE THE LENGTH OF THE CANTILEVER.) NOTE THAT THE TOTAL MAXIMUM CALCULATED FLOOR SYSTEM DEFECTION WILL BE THE SUM OF THE DEFLECTIONS OF THE SUPPORTED ELEMENTS IN A BAY.

THE CALCULATED DEFLECTION FOR INDIVIDUAL MEMBERS SUPPORTING MASONRY DO NOT EXCEED L/600 FOR DESIGN LOADS APPLIED AFTER THE INSTALLATION OF THE MASONRY.

2.10. SPECIAL INSPECTIONS:

2.10.1. THE STRUCTURAL TESTING/INSPECTION AGENCY WILL PERFORM SPECIAL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE.

2.10.2. SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE ARE REQUIRED FOR STRUCTURAL COMPONENTS AND ASSEMBLIES WHICH ARE NOT FABRICATED AT THE CONSTRUCTION JOB SITE INCLUDING BUT NOT LIMITED TO FLOOR AND ROOF TRUSSES AND JOISTS OF WOOD AND STEEL MATERIALS, STRUCTURAL STEEL FRAMING, AND PRECAST CONCRETE, JOISTS, BEAMS, COLUMNS, SLABS, WALLS AND CLADDING.

2.10.3. SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE MAY BE WAIVED FOR ITEMS WHICH ARE PRODUCED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND BY PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE WHICH STATES THAT THE FABRICATION WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

2.10.4. THE PROJECT CONTRACTOR WILL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PERFORM INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE DURING CONSTRUCTION OF THE PROJECT. DOCUMENTATION THAT SUMMARIZES THE QUALIFICATION AND CREDENTIALS OF EACH SPECIAL INSPECTOR AND DEMONSTRATES COMPETENCE FOR INSPECTION OF EACH PARTICULAR TYPE OF CONSTRUCTION REQUIRING SPECIAL INSPECTION SHALL BE SUBMITTED TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

2.10.5. APPROVED SPECIAL INSPECTORS SHALL FURNISH INSPECTION REPORTS TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE AND TO THE OWNER WHICH INDICATES THAT THE WORK INSPECTED WAS DONE IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. A FINAL REPORT WHICH DOCUMENTS THE RESULTS OF THE SPECIAL INSPECTIONS PERFORMED INCLUDING CORRECTION OF ANY DISCREPANCIES IDENTIFIED DURING INSPECTION SHALL BE SUBMITTED PERIODICALLY AT A FREQUENCY APPROVED BY THE CHIEF COMMERCIAL BUILDING INSPECTOR PRIOR TO CONSTRUCTION.

2.11. NO PROVISIONS HAVE BEEN MADE FOR FUTURE VERTICAL EXPANSION. DESIGN HAVE BEEN COMPLETED TO INCLUDE A FUTURE BAY TO THE EAST SIMILAR TO THE CURRENT EAST BAY.

3. FOUNDATION NOTES

3.1. FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY STRATA, REPORT NUMBER TF24239E DATED JANUARY 7, 2025. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT TO THOSE ASSUMED FOR DESIGN.

3.2. CONTRACTOR MAY EXAMINE THE SITE AND MAKE THEIR OWN SUBSURFACE EXPLORATION AT NO ADDITIONAL COST TO THE OWNER. NOTIFY OWNER PRIOR TO MAKING ANY SUBSURFACE EXPLORATIONS.

3.3. NOTIFY THE OWNER/STRUCTURAL ENGINEER 48 HOURS PRIOR TO THE BEGINNING OF EXCAVATION WORK.

3.4. LOCATE EXISTING UTILITIES BY CAREFUL HAND EXCAVATION. IF UTILITIES ARE TO REMAIN IN PLACE, PROVIDE PROTECTION FROM DAMAGE DURING CONSTRUCTION OPERATIONS. COOPERATE WITH OWNER AND UTILITY COMPANIES IN KEEPING RESPECTIVE SERVICES AND FACILITIES IN OPERATION. DO NOT INTERRUPT EXISTING UTILITY SERVICES AND FACILITIES UNLESS WRITTEN PERMISSION IS GIVEN BY THE OWNER AND THEN ONLY AFTER TEMPORARY UTILITY SERVICES HAVE BEEN PROVIDED.

3.5. SHOULD UNCHARTED OR INCORRECTLY CHARTED PIPING OR OTHER UTILITIES BE ENCOUNTERED DURING EXCAVATION, CONSULT WITH OWNER IMMEDIATELY FOR DIRECTIONS.

3.6. REPAIR DAMAGED SERVICES TO SATISFACTION OF UTILITY OWNER.

3.7. FOOTINGS HAVE BEEN DESIGNED WITH AN ALLOWABLE BEARING CAPACITY OF 5000 PSF.

3.8. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.

3.9. NO FOOTINGS SHALL BEAR ON ROCK. UNDERCUT ROCK A MINIMUM OF 2 FEET

3.10. UNDERCUT THE ENTIRE BUILDING AREA TO THE EXTENT FIVE FEET BEYOND THE BUILDING ENVELOPE AND A DEPTH OF SIX FEET AND REPLACE WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE GEOTECHNICAL REPORT.

3.11. GROUNDWATER MAY BE ENCOUNTERED DURING THE FOUNDATION EXCAVATION. PROVIDE A SYSTEM FOR CONTROLLING THE GROUNDWATER TO A LEVEL AT LEAST THREE FEET BELOW THE LOWEST POINT OF THE EXCAVATION.

3.12. KEEP EXCAVATIONS DRY BY SLOPING THE GROUND AWAY FROM HOLES AND TRANCHES. AREAS FOUND TO BE SOFT OR SLUMPING SHALL HAVE THE SOFT SOIL REMOVED AND REPLACES WITH THE SPECIFIED STRUCTURAL FILL AND COMPACTED AS OUTLINED HEREIN.

3.13. GRANULAR STRUCTURAL FILL

3.13.1. ALLOWABLE USES: GENERAL STRUCTURAL FILL AND OVER-EXCAVATIONS

3.13.2. SOIL CLASSIFIED AS GW, GP, GP-GM, SP AND SP-SM ACCORDING TO THE USCS,AND MEETING THE GRADATION OF 6- INCH MINUS AND LESS THAN 10% PASSING #200 SIEVE

3.13.3. SOIL MEETING REQUIREMENTS STATED IN THE LATEST EDITION OF THE IDAHO STANDARD FOR PUBLIC WORKS CONSTRUCTION (ISPWC), SECTION 801 - AGGREGATE SUBBASE, WITH A MAXIMUM PARTICLE SIZE OF 6 INCHES

3.14. GENERAL STRUCTURAL FILL

3.14.1. ALLOWABLE USES: GENERAL SITE GRADING

3.14.2. STRUCTURAL FILL SHALL CONSIST OF SOIL CLASSIFIED AS GW, GP, GP-GM, GM, SW, SP, SP-SM, SM, SP-SC, SC,ML OR CL ACCORDING TO THE USCS.

3.14.3. MAXIMUM PARTICLE SIZE MUST BE LESS THAN 6 INCHES

3.14.4. SOIL CONSISTING OF INERT EARTH MATERIALS WITH LESS THAN 3% ORGANICS OR OTHER DELETERIOUS SUBSTANCES (WOOD, METAL, PLASTIC, WASTE, ETC.)

3.15. PLACEMENT OF FILL/SUBBASE

3.15.1. DO NOT PLACE ON SUBGRADE THAT CONTAINS FROST, MUD, OR IS FROZEN.

3.15.2. STRUCTURAL FILL SHALL BE PLACED AND COMPACTED IN 10-INCH MAXIMUM THICK LOOSE LAYER.

3.15.3. STRUCTURAL FILL SHALL CONTAIN NO ORGANIC MATERIAL AND BE APPROVED BY A GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT. STRUCTURAL FILL UNDER SLABS AND WITHIN 5'-0" OF THE BUILDING FOOTPRINT SHALL BE PLACED IN LIFTS OF THICKNESS DETERMINED BY THE INDEPENDENT TESTING AGENCY AND COMPACTED TO AT LEAST 95% OF ITS MODIFIED PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D1557. THE TOP 12" SUB-BASE UNDER SLABS ON GRADE SHALL BE COMPACTED TO AT LEAST 98% OF ITS MODIFIED PROCTOR MAXIMUM DRY DENSITY. ALL BACKFILL, COMPACTION AND PROOF ROLLING OPERATIONS SHALL BE OBSERVED BY AN INDEPENDENT TESTING LABORATORY.

3.15.4. SLABS-ON-GRADE SHALL BE PLACED ON A 4" GRANULAR BASE, COMPACTED TO 95% OF ITS MODIFIED PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D1557, AND COVERED WITH A 10 MIL CONTINUOUSLY SEALED VAPOR BARRIER. THE BASE FOR SLABS-ON-GRADE SHALL BE INSPECTED BY A GEOTECHNICAL ENGINEER PRIOR TO EACH PLACEMENT OF CONCRETE.

3.15.5. BACKFILL SHALL NOT BE PLACED AGAINST EXTERIOR OR RETAINING WALLS UNTIL THE WALLS HAVE ACHIEVED THEIR DESIGN STRENGTH AND THEIR LATERAL SUPPORT ELEMENTS ARE INSTALLED. PROVIDE ADEQUATE DRAINAGE AT BASEMENT AND RETAINING WALLS.

3.15.6. COMPACT BACKFILL BEHIND WALLS SHALL BE PLACED IN LAYERS OF 10_ INCHES.

3.15.7. COMPACT BACKFILL BEHIND WALLS TO 95% OF THE MAXIMUM DRY DENSITY AS MEASURED BY THE MODIFIED PROCTOR, ASTM D1557

3.16. REMOVE EXCESS EXCAVATED MATERIALS FROM THE JOB SITE AND LEAVE SITE IN "CLEAN" CONDITION UPON COMPLETION

3.17. TESTING/INSPECTION AGENCY SHALL PERFORM THE FOLLOWING QUALITY RELATED ITEMS

3.17.1. VERIFY STRUCTURAL FILL COMPLIES WITH THE GENERAL NOTES AND GEOTECHNICAL REPORT, IF APPLICABLE.

3.17.2. DETERMINE PARTICLE SIZE, LIQUID LIMIT, PLASTIC LIMIT, PLASTICITY INDEX, AND MAXIMUM DENSITY OF EACH SOIL TYPE.

3.17.3. PERFORM A SUFFICIENT NUMBER OF FIELD DENSITY TESTS TO VERIFY COMPACTION OF STRUCTURAL FILL. AS A MINIMUM, PERFORM ONE TEST PER LIFT FOR EVERY 1500 SQUARE FEET OF FILL PLACED.

3.18. FOOTINGS SHALL BE CENTERED ABOUT COLUMN LINES UNLESS NOTED OTHERWISE.

3.19. ALL FOOTINGS AND TURN DOWN SLAB EDGES SHALL PENETRATE TO A MINIMUM DEPTH OF 24" BELOW FINISHED GRADE.

3.20. SIDES OF FOUNDATIONS SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING.

3.21. FOUNDATIONS POURED AGAINST EARTH REQUIRE THE FOLLOWING PRECAUTIONS:

3.21.1. SLOPE SIDES OF EXCAVATIONS AS APPROVED BY A GEOTECHNICAL ENGINEER.

3.21.2. CLEAN UP SLOUGHING PRIOR TO AND DURING CONCRETE PLACEMENT.

3.21.3. WHERE STEPPED FOUNDATIONS ARE NECESSARY, THE STEPS SHALL NOT BE STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL.

4. CONCRETE REINFORCEMENT NOTES

4.1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 DEFORMED BAR, UNLESS NOTED OTHERWISE.

4.2. REINFORCING STEEL SUBJECT TO WELDING SHALL CONFORM TO ASTM A706, GRADE 60, DEFORMED BAR, USE ONLY LOW HYDROGEN (E80XX) ELECTRODES, AND WELD IN ACCORDANCE WITH AWS D12.1 AND OR AWS D1.4. WELD ONLY AS INDICATED.

4.3. WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185 AND HAVE MINIMUM SIDE AND END LAPS OF 8". WWR SHALL BE PROVIDED IN FLAT SHEETS ONLY.

4.4. BEFORE PLACING CONCRETE, CLEAN REINFORCEMENT OF FOREIGN PARTICLES AND COATINGS.

4.5. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES, SPACING, LOCATIONS, AND QUANTITIES OF REINFORCING STEEL AND WIRE FABRIC, BENDING AND CUTTING SCHEDULES, SPLICE LENGTHS, STIRRUP SPACING, SUPPORTING AND SPACING DEVICES. DETAIL REINFORCING STEEL IN ACCORDANCE WITH ACI 315 AND CRSI STANDARDS. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.

4.6. SPLICES SHALL BE CLASS B IN ACCORDANCE WITH ACI 318, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.

4.7. TENSILE SPLICERS SHALL BE CAPABLE OF DEVELOPING 125% OF THE REINFORCING STEEL ASTM SPECIFIED MINIMUM YIELD STRENGTH. TENSION COUPLERS MAY BE USED AND INSTALLED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.

No.	Date	By	Revision	Ad. No.
D	2025.07.02	DV	ADDENDUM 1	
C	2025.06.20	DV	OWNER COMMENTS	
B	2025.04.25	SW	ISSUED FOR BID	
A	2025.04.11	SW	ISSUED FOR APPROVAL (70% CHECKSET)	

PRELIMINARY
NOT FOR
CONSTRUCTION
02 JULY 2025

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Title: AMALGAMATED SUGAR COMPANY EVAPORATOR PROJECT
GENERAL NOTES AND STRUCTURAL SPECIFICATIONS

Amalgamated Sugar

AMALGAMATED SUGAR COMPANY
2320 ORCHARD DRIVE EAST
TWIN FALLS, ID 83301

Date: 2025.04.10
Chkd By: S. WILSON
Dsgn By: D. VIELE
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